

Oregon Building Codes Division
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2005 Oregon Residential Specialty Code Errata (As of 03/27/06)

Strike through denotes deleted language.
Underline denotes added language

1. Make the following corrections to Section R102.7.1 (last exception) (page 1-3), as follows;

Exception: Water heaters installed in garages must comply with Sections M1307.3 and ~~P2810.6~~ P2810.3.

2. Make the following corrections to Section R301.2.2.4.1 (page 3-13) as follows;

R301.2.2.2.4.1 Height limitations. Wood framed buildings shall be limited to three stories above grade or the limits given in Table R602.10.~~3(1)~~ or Table R602.10.3(2). Cold-formed steel framed buildings shall be limited to two stories above grade in accordance with COFS/PM. Mezzanines as defined in Section R202 shall not be considered as stories.

3. Make the following corrections to Table R301.5 (page 3-14) as follows;

**TABLE R301.5
 MINIMUM UNIFORMLY DISTRIBUTED
 LIVE LOADS
 (In pounds per square foot)**

USE	LIVE LOAD
Attics with storage ^b	20
Attics without storage	10
Decks ^e	40
Exterior balconies	60
Fire escapes	40
Guardrails and handrails ^d	200
Guardrails in-fill components ^f	200 <u>50</u>
Passenger vehicle garages ^a	50 ^a
Dwelling units	40
Stairs	40 ^c

4. Make the following corrections to Section R311.4.3 (page 3-21) as follows;

R311.4.3 Landings at doors. There shall be a floor or landing on the interior side of each exterior door. There shall be a floor or landing on the exterior side of the required exit door described in Section ~~R311.4~~ R311.4.1.

5. Make the following corrections to Section R323.1 (page 3-29) as follows;

R323.1 General. Buildings and structures constructed in flood hazard areas (including A or V Zones) as identified by the local jurisdiction shall be designed and constructed in accordance with the provisions contained in this section. For the purposes of Section R323, the required elevation of construction elements shall be a minimum of 1 foot (305 mm) above the design flood elevation unless increased by the local municipality under the authority of National Flood Insurance Program incorporated in 423 U.S.C. 4001-4128.

Exception: All buildings and structures in identified floodways as established ~~identified~~ by the local jurisdiction shall be designed and constructed as stipulated in the *Oregon Structural Specialty Code* or equivalent design methods based on nationally recognized standards.

6. Make the following corrections to Section R323.1.8 (page 3-30) as follows;

R323.1.8 Manufactured housing. New or replacement manufactured housing shall be elevated in accordance with ~~Section R323.2 and the anchor and tie-down requirements of Sections 3-2.4 AE604 and AE605 of the Oregon Manufactured Dwelling and Park Specialty Code Appendix E shall apply.~~ The foundation and anchorage of manufactured housing to be located in identified flood ways as established by the local jurisdiction ~~in Table R301.2(1)~~ shall be designed and constructed in accordance with the applicable provisions in Section 3-2.4.2 of the Oregon Manufactured Dwelling and Park Specialty Code. *International Building Code.*

7. Make the following corrections to Section R408.2 Exception 5 (page 4-24) as follows;

5. Ventilation openings are not required when the ground surface is covered with an approved ground cover material, the space is supplied with conditioned air and the perimeter walls are insulated in accordance with Section ~~N1102.1.7~~ N1104.2.5.

8. Make the following corrections to Section R408.6 (page 4-24) as follows;

R408.6 Flood resistance. For buildings located in areas prone to flooding as established by the local jurisdiction: ~~the walls enclosing the underfloor space shall be provided with flood openings in accordance with Section R327.2.2.~~

1. Walls enclosing the underfloor space shall be provided with flood openings in accordance with Section ~~R323.2.2~~ R327.2.2.
2. The finished ground level of the underfloor space shall be equal to or higher than the outside finished ground level.

Exception: Underfloor spaces that meet the requirements of FEMA/FIA TB 11-1.

9. Make the following corrections to Section R502.3.1 & .2 (page 5-1) as follows;

R502.3.1 ~~Sleeping areas and~~ Attic joists. Table R502.3.1(1) shall be utilized to determine the maximum allowable span of floor joists that support ~~sleeping areas and~~ attics that are accessed by means of a fixed stairway provided that the design live load does not exceed 30 psf (1.44 kN/m²) and the design dead load does not exceed 10 psf (0.48 kN/m²). The allowable span of ceiling joists that support attics utilized for limited storage or no storage shall be determined in accordance with Section R802.4.

R502.3.2 Other floor joists. Table R502.3.1(2) shall be utilized to determine the maximum allowable span of floor joists that support all areas of the building, other than ~~sleeping and~~ attics, provided that the design live load does not exceed 40 psf (1.92 kN/m²) and the design dead does not exceed 10 psf (0.48 kN/m²).

10. Make the following corrections to Table R503.2.1.1(1) (page 5-13) as follows;

Span Rating	Minimum nominal panel thickness (inches)	Maximum Span (inches) ^d		Load (pounds per square foot, at maximum span)		Maximum Span (inches)
		With edge support	Without edge support	Total load	Live load	
Sheathing ^e		Roof ^f				Subfloor ^j
48/24	²³ / ₃₂ , ⁷ / ₄₈ ³ / ₄	48	36	45	35	24

11. Make corrections to the heading of Table R602.3.1 (page 6-9) as follows;

TABLE R602.3.1 – continued
MAXIMUM ALLOWABLE LENGTH OF WOOD WALL STUDS EXPOSED TO WIND SPEEDS OF 110 ~~100~~ MPH OR LESS
IN SEISMIC DESIGN CATEGORIES A, B, C, ~~AND D₁~~ AND D₂

12. Make corrections to Section R602.10.1.1 (page 6-13), Exception item 1 as follows;

R602.10.1.1 Spacing. Spacing of braced wall lines in structures located in Seismic Design Categories D₁ and D₂ shall not exceed 35 feet (10,668 mm) on center in both the longitudinal and transverse directions in each story.

Exception: Spacing of braced wall lines in one or two story buildings located in Seismic Design Category D₁ and D₂, not exceeding 50 feet shall be permitted where:

1. The wall bracing provide equals or exceeds the amount of bracing required by Table R602.10.3(1) or Table R602.10.3(2) multiplied by a factor equal to the braced wall line spacing divided by 35 feet, and
2. The length-to-width ratio for the floor/wall diaphragm does not exceed 3:1.

13. Make corrections to Table R602.10.3(2) (page 6-15), footnotes “g” and “h”. The table referenced in these two footnotes should read; “Table R602.10.3(1)”

14. Make corrections to Section R602.10.2, last paragraph (page 6-16) as follows;
R602.10.2 Cripple wall bracing.

In Seismic Design Category D₂, exterior framed walls supporting three stories are not permitted. Cripple walls shall be braced in accordance with Tables R602.10.3(1) or R602.10.3(2).

15. Make corrections to R602.10.5 (page 6-16) as follows;

“.....edges nailed. Wood structural panel sheathing shall be installed at corners in accordance with Figure R602.10.5. The bracing amounts in Table R602.10.3(1) for Method 3 shall be permitted to be multiplied by a factor of 0.9 for walls with a maximum opening height that does not exceed 85 percent of the wall height or a factor of 0.8 for walls with a maximum.....”.

16. Make corrections to R602.10.7 (page 6-19) as follows;

R602.10.7 Panel joints. All vertical joints of panel sheathing shall occur over studs. Horizontal joints in braced wall panels shall occur over blocking of a minimum of 1½ inch (38 mm) thickness.

Exception: Blocking is not required behind horizontal joints in Seismic Design Categories A and B and detached dwellings in Seismic Design

Category C when constructed in accordance with R602.10.3, Braced-wall-panel construction method 3 and Table R602.10.3(1), method 3, or where permitted by the manufacturer's installation requirements for the specific sheathing material.

17. Change R602.10.11.2 (page 6-20) to read as follows; (The whole section has been added to help clarify)

R602.10.11.2 Three or more horizontally attached units. Braced panels that are not located at the end of a braced wall line shall comply with the following provisions:

1. In walls sheathed in accordance with table R602.10.3(2), The end of the braced wall panel closest to the corner shall have a tie-down device fastened to the stud at the edge of the braced wall panel closest to the corner and to the foundation or an equivalent cross section of stud in the wall below. in the first of a two-story building or second of a three-story building, the tie-down device shall be capable of providing an uplift allowable design value of at least 1,800 pounds (816.5 kg). In the first of a three-story building, the tie-down device shall be capable of providing an uplift allowable design value of at least 3,000 pounds (1360.8 kg). The tie-down device shall be installed in accordance with the manufacturer's recommendations.

2. In walls sheathed in accordance with Table R602.10.3(1), the end of each side of the braced panel closest to the corner shall have a tie-down device fastened to each end stud and to the foundation or an equivalent cross section of stud in the wall below. In the first of a two-story building or second of a three-story building, the tie-down device shall be capable of providing an uplift allowable design value of at least 1,800 pounds (816.5 kg). In the first of a three-story building, the tie-down device shall be capable of providing an uplift allowable design value of at least 3,000 pounds (1360.8 kg). The tie-down device shall be installed in accordance with the manufacturer's recommendations.

No tie-down device is required for a one-story building, the top of a two or top of a three story building.

Exception: The required uplift capacities for tie-down devices may be reduced by 25% for braced panels installed within Seismic Design Category C except in areas exposed to Columbia River Gorge as per Figure R301.2(4).

The provisions in this second paragraph, and the exception (shown underlined) were intended to apply to both items 1 and 2, but was inadvertently added to item 2 during printing.)

18. Make the following corrections to Section R602.11.1 (page 6-20) to read as follows;

R602.11.1 Wall anchorage. Braced wall line sills shall be anchored to concrete or masonry foundations in accordance with Sections ~~R403.1.6~~ **R403.1.8** and R602.11. For buildings located in Seismic Design Categories D₁ and D₂, plate washers, a minimum of $\frac{3}{16}$ inch by 2 inches by 2 inches (6.4 mm by 51 mm by 51 mm) or $\frac{1}{4}$ inches in diameter in size, shall be provided between the foundation sill plate and the nut.

Exception: Detached One-and Two-family dwellings in Seismic Design Category ~~D~~ **D₁**.

19. Make the following corrections to Table R703.4 (page 7-6), footnote **j** as follows;

j. Three-eighths-inch plywood shall not be applied directly to studs spaced greater than 16 inches on center when long dimension is parallel to studs. One-half-inch plywood shall not ~~may~~ be applied directly to studs spaced greater than 24 inches on center. The stud spacing shall not exceed the panel span rating provided by the manufacturer unless the panels are installed with the face grain perpendicular to studs or over sheathing approved for that stud spacing.

20. Make the following corrections to Section N1101.2.1 (page 11-1), as follows;

N1101.2.1 Alteration and repair. Alterations and repairs affecting energy conservation measures shall conform to the requirements specified in this chapter.

Alterations or repairs which affect components of existing conditioned spaces regulated in this chapter shall comply with this chapter.

Exception: The minimum component requirements as specified in Footnote ~~e~~ **d** of Table N1104.1(2) may be used to the maximum extent practical.

21. Section G2415.14.1 (page 24-24) should read;

G2415.14.1 Limitations. Plastic pipe shall be installed outside underground only. Plastic pipe shall not be used within or under any building or ~~slab~~ building slab or be operated at

22. Make the following correct to Appendix G (page G-1);

AG103.1 In-ground pools. ~~Not adopted by the State of Oregon.~~ In-ground pools shall be designed and constructed in accordance with ANSI/NSPI-5 as listed in Section AG107.1.

23. Make corrections to Section AN109.4.1, in Appendix N (page N-17) as follows;

AN109.4.1 Local adoption. The provisions of ~~AN109.3~~ AN109.4.2 or AN109.4.3 apply only when specifically adopted by the local authority having jurisdiction.

24. **Appendix K** (page K-1) is adopted as part of this code, but was inadvertently left out of the printing of the code. (A printable copy of Appendix K is included)

APPENDIX K
SOUND TRANSMISSION

SECTION AK101

GENERAL

AK101.1 General. Wall and floor-ceiling assemblies separating dwelling units shall provide airborne sound insulation for walls, and both airborne and impact sound insulation for floor-ceiling assemblies.

SECTION AK102

AIRBORNE SOUND

AK102.1 General. Airborne sound insulation for wall and floor-ceiling assemblies shall meet a Sound Transmission Class (STC) rating of 45 when tested in accordance with ASTM E 90.

SECTION AK103

STRUCTURAL-BORNE SOUND

AK103.1 General. Floor/ceiling assemblies between dwelling units or between a dwelling unit and a public or service area within a structure shall have an Impact Insulation Class (IIC) rating of not less than 45 when tested in accordance with ASTM E 492.

SECTION AK104

REFERENCED STANDARDS

ASTM E90-99 Test Method for Laboratory
Measurement of Airborne Sound Transmission
Loss of Building Partitions and Elements..... AK102

ASTM E 492-90 (1996)e Specification for
Laboratory Measurement of Impact Sound
Transmission through Floor-ceiling Assemblies
Using the Tapping Machine AK103